Nalcor Energy Response to Grant Thornton Question 6.2

23 May 2018

Question:

In reference to the Lower Churchill Project Phase 1 Decision Gate 3 Support Package, please confirm the design criterion for the overland HVDC Transmission line, (i.e., return period).

Response:

The design criteria for the Labrador-Island Transmission Link (LITL) are discussed in detail in the response to Request for Information NP-NLH-004, "Investigation and Hearing into Supply Issues and Power Outages on the Island Interconnected System", attached and also found at: http://www.pub.nl.ca/applications/IslandInterconnectedSystem/files/rfi/NP-NLH-004.pdf

The design criteria for the LITL were developed following the principles outlined in CAN/CSA 22.3 No.60826. They were developed giving consideration to reference data from the standard, operational experience and identified operational risks, as well as model data for unique conditions (rime ice) not incorporated in the standard. These design criteria are as presented in Exhibit 97 (Amended) for the Muskrat Falls PUB review, attached, and also found at NAL0074240 and: http://www.pub.nl.ca/applications/MuskratFalls2011/files/exhibits/Exhibit97-AppendixA-Rev1.pdf

The as-designed structures were evaluated against the following loadings:

- a) CSA 150-year ice loadings for the line section off the Avalon Peninsula.
- b) CSA 500-year ice loadings for the route on the Avalon Peninsula.
- c) CSA 150-year wind loadings for the line section off the Avalon Peninsula.
- d) CSA 500-year wind loadings for the line section on the Avalon Peninsula.

No structure was loaded beyond 100% of its design structural capacity in any of these scenarios.

The design criteria for rime ice accretion are beyond 500-year return period loads predicted by rime ice accretion models developed by Landsvirken Power, and the design criteria for wind are beyond 500-year CSA return period winds in rime zones.

The as-designed structures were evaluated against proposed combined wind and ice criteria in IEC Standard 60826 in the following conditions:

a) Extreme wind (72% or 85% of reference wind as applicable for 150-year and 500-year zones respectively) combined with 40% of reference ice load; and,

b) Maximum ice (150 or 500-year loads as applicable for 150-year and 500-year zones respectively) combined with 40% of reference wind speed.

No structure was loaded beyond 100% of its as-designed structural capability in any two of these scenarios.

Page 3

NP-NLH-004 Island Interconnected System Supply Issues and Power Outages

Page 1 of 57

1	Q.	Reference: Public Utilities Board Muskrat Falls Review, Manitoba Hydro
2		International: Report on Two Generation Expansion Alternatives for the Island
3		Interconnected Electrical System, January 2012, page 11.
4		"Design Loading Criteria – Nalcor has selected a 1:50-year
5		reliability return period (basis for design loading criteria)
6		for the HVdc transmission line, which is inconsistent with
7		the recommended 1:500-year reliability return period
8		outlined in the International Standard CEI/IEC 60826:2003
9		with Canadian deviations in CSA Standard CAN/CSA-C22.3
10		No. 60826:06, for this class of transmission line without an
11		alternate supply."
12		Please confirm the return period of climatic loads used in the design of the Labrador
13		 Island HVdc Link and provide all the detailed ice and wind weather cases as well
14		as suspension tower load cases, including the mathematical calculations supporting
15		them.
16		
17	Α.	In order to provide context regarding the quotation in the question, the Board's
18		review of Muskrat Falls in 2011 was undertaken prior to the completion of detailed
19		engineering for the Labrador Island Transmission Link (LITL) and considered
20		feasibility study inputs rather than completed engineering. As well, the availability
21		of the Maritime Link as a support for an alternate supply source was also not a
22		consideration as part of the review. Since then, the design criteria for the LITL have
23		been finalized and its design has been completed.
24		
25		As a result, the statement that "Nalcor has selected a 1:50-year reliability return
26		period" is not reflective of the final climatic loading design criteria for the LITL, the

Page 4

	NP-NLH-004 Island Interconnected System Supply Issues and Power Outages
	Page 2 of 57
1	results of a comparison of LITL design loads to CSA reference loads, nor its
2	as-designed capability.
3	
4	This response provides clarification of the design criteria, and also clarification of
5	the capability of the line after the completion of the design process.
6	
7	The following conclusions are supported in this response:
8	• The design criteria for the LITL were developed following the principles
9	outlined in CAN/CSA 22.3 No. 60826. They were developed giving
10	consideration to reference data from the standard, operational experience
11	and identified operational risks, as well as model\data for unique conditions
12	(rime ice) not incorporated in the standard.
13	• In addition to the design criteria established for the LITL, the as-designed
14	structures were evaluated against the following loadings:
15	a) CSA 150-year ice loadings for the line section off the Avalon Peninsula.
16	b) CSA 500-year ice loadings for the route on the Avalon Peninsula.
17	c) CSA 150-year wind loadings for the line section off the Avalon Peninsula.
18	d) CSA 500-year wind loadings for the line section on the Avalon Peninsula.
19	No structure was loaded beyond 100% of its as-designed structural capacity
20	in any of these scenarios.
21	• The design criteria for rime ice accretion are beyond 500-year return period
22	loads predicted by rime ice accretion models developed by Landsvirkjun
23	Power (LVP), and the design criteria for wind are beyond 500-year CSA
24	return period winds in rime zones.
25	• The as-designed structures were evaluated against proposed combined wind
26	and ice criteria in International Electrotechnical Commission (IEC) Standard
27	60826 in the following conditions:

Page 5

	NP-NLH-004
	Island Interconnected System Supply Issues and Power Outages
1	a) Extreme wind (72% or 85% of reference wind as applicable for 150-
2	year and 500-year zones respectively) combined with 40% of
3	reference ice load; and
4	b) Maximum ice (150 or 500-year rime or glaze loads as applicable for
5	150-year and 500-year zones respectively) combined with 40% of
6	reference wind speed.
7	
8	No structure was loaded beyond 100% of its as-designed structural capacity
9	in any of these scenarios.
10	
11	Further details are provided in this response below supporting the conclusion that
12	LITL has been designed in accordance with applicable standards and that it meets
13	climatic loading conditions commensurate with its important role in the Island
14	Interconnected System.
15	
16	The climatic loading criteria for the LITL were established using the principles
17	outlined in CSA standard CAN/CSA 22.3 No. 60826. Guidance on reliability levels is
18	provided for in the IEC Standard 60826, which is incorporated into CAN/CSA 22.3
19	No. 60826. Guidance to designers is provided in Section A.1.2.5 of the IEC standard,
20	as presented below:
21	
22	A.1.2.5 Selection of reliability levels
23 24 25	Transmission lines can be designed for different reliability levels (or classes) depending on local requirements and the line duties within a supply network.
26 27 28	Designers can choose their reliability levels either by calibration with existing lines that have had a long history of satisfactory performance or by optimisation methods found in technical literature.
29 30 31 32	In all cases, lines should at least meet the requirements of a reliability level characterized by a return period of loads of 50 years (level 1).

NP-NLH-004

Island Interconnected System Supply Issues and Power Outages
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	Page 4 of 57
1 2 3	An increase in reliability above this level could be justified for more important lines of the network as indicated by the following guidelines:
4 5 6 7 8	 It is suggested to use a reliability level characterised by return periods of 150 years for lines above 230 kV. The same is suggested for lines below 230 kV which constitute the principal or perhaps the only source of supply to a particular electric load (level 2).
9 10 11 12	 Finally, it is suggested to use a reliability level characterised by return periods of 500 years for lines, mainly above 230 kV which constitute the principal or perhaps the only source of supply to a particular electric load. Their failure would have serious consequences to the power supply.
13 14 15 16 17	The applications of the reliability for overhead lines, including corresponding voltage levels, may be set differently in individual countries depending on the structure of the grid and the consequences of line failures. The impacts on other infrastructure installations such as railroads and motorways should be considered as well.
18 19 20 21	When establishing national and regional standards or specifications, decisions on the reliability level should be made taking into consideration also the experience with existing lines.
22	The standard indicates a minimum reliability level characterized by a 50-year return
23	period, and also identifies circumstances under which a higher reliability level
24	characterized by longer return periods could be warranted. This standard takes the
25	form of a recommended practice.
26	
27	The line design for the LITL was undertaken in accordance with the principles
28	outlined in CAN/CSA 22.3 No. 60826. Multiple sources of information were used to
29	provide wind, ice, and combined loading along the transmission line route. This is
30	in accordance with Section 4.1 of the standard:
31	
32	"The climatic data provided in this Standard may be augmented by
33	reliable regional or local data where available."
34	
35	The multiple sources of data used to establish load conditions for the LITL
36	are listed below:

Page 7

	NP-NLH-004
	Island Interconnected System Supply Issues and Power Outages
1	Page 5 of 57 a) Reference wind and ice loading as provided in the CSA standard;
2	b) A study undertaken by Kathleen Jones of the Cold Regions Research and
2	Engineering Laboratonu ¹
5	Engineering Laboratory,
4	c) Hydro's applicable nearly 50-year operating history along the transmission
5	line route; ² and
6	d) A study undertaken by Landsvirkjun Power which evaluated rime (or in-
7	cloud) ice loadings, which are a design consideration along the LITL's route,
8	but are not addressed by CAN/CSA 22.3 No. 60826. ³
9	
10	Some design aspects warranted special engineering consideration:
11	a) Hydro's operating experience on the Avalon Peninsula and previous icing
12	events resulted in increased design loading; ⁴
13	b) A conservative approach was taken to selection of rime ice criteria in locales
14	where rime accretion is prevalent, as discussed later in this response; and
15	c) Routing decisions were taken to avoid high country and exposed areas
16	throughout the route.
17	
18	These approaches were all taken to improve the reliability of the LITL and to
19	mitigate operating risk.
20	
21	The standard also recognizes a level of variability in performance, and IEC 60826
22	discusses reliability levels and associated failure probabilities in Table A.2 of the
23	Annex to the standard:

 ¹<u>http://www.pub.nl.ca/applications/MuskratFalls2011/files/exhibits/Exhibit96.pdf</u>.
 ²Particularly on Newfoundland's Avalon Peninsula.
 ³<u>http://www.pub.nl.ca/applications/MuskratFalls2011/files/exhibits/Exhibit95.pdf</u>.
 ⁴ Ultimately, the 2010 version of the CSA standard further increased loadings.

Page 8

NP-NLH-004 Island Interconnected System Supply Issues and Power Outages

Page 6 of 57

Table A.2 – Relationship between reliability levels and return periods of limit loads

Return period of limit loads	50	150	500	
Yearly minimum reliability	Ps1	0,98 to 0,99	0,993 to 0,997	0,998 to 0,999
Yearly failure probability	P_{f1}	0,02 to 0,01	0,0067 to 0,0033	0,002 to 0,001
Reliability during 50 years life cycle	P _{s50}	0,36 to 0,61	0,71 to 0,86	0,90 to 0,95
Theoretical probability of failure during 50 years life cycle	P _{f50}	0,64 to 0,39	0,29 to 0,14	0,10 to 0,05

1

In considering the probability of a failure, the Annex suggests that theoretically,
there is a 39% to 64% chance that a line failure will occur over the 50-year design
life of the line with a 50-year return period, a 14% to 29% probability with a
150-year return period, and a 5% to 10% probability at a 500-year return period.
On an annual basis, the probability of a failure is 1% to 2% with a 50-year return
period, 0.3% to 0.7% with a 150-year return period, and a 0.1% to 0.2% probability
with a 500-year return period.

9

In the context of the Island Interconnected System, the LITL will be routed adjacent
to the existing 230 kV corridor from Sunnyside to Soldier's Pond. Routing
alternatives are limited because of the topography of the Avalon and limitations
imposed by existing developments in the most populated area of the Province. This
constraint warrants comparing the line design to a 500-year return period in this
important corridor.

16

West of Sunnyside, the LITL takes a route that is independent of the 230 kV corridor
from Bay d'Espoir. As a result, the two major transmission systems serving the
Avalon Peninsula are geographically separated from each other. Given the
separation and the independent generation available on each system, a 150-year
return period is appropriate for the remainder of the LITL route.

Page 9

	NP-NLH-004
	Island Interconnected System Supply Issues and Power Outages
	Page 7 of 57
1	Consequently, the structural capability of the LITL will be compared to CSA 500-year
2	return period loads on the Avalon Peninsula and for CSA 150-year return periods on
3	the remainder of the LITL route.
4	
5	Given the varied conditions through which the LITL is routed, 11 different
6	meteorological design zones were identified to address varying conditions that exist
7	along the line's 1,100 km length. The overall transmission line route and
8	corresponding meteorological design zones for the Labrador and Island portions of
9	the LITL are shown in Figures 1, 2, and 3.
10	



Figure 1 – LITL Transmission Route

Page 10







Page 11

CIMFP Exhibit P-03188

NP-NLH-004 Island Interconnected System Supply Issues and Power Outages

Page 9 of 57





Page 12

NP-NLH-004 Island Interconnected System Supply Issues and Power Outages

Page 10 of 57

- 1 These 11 zones are an extension of the approach used in the design of the original
- 2 Bay d'Espoir 230 kV system, where "normal" and "ice" design zones were
- 3 established for the Island.⁵ The following categories are used for LITL:
- 4

Description	Zone
"Normal" conditions found in the interior of Labrador	1,8b, 10
and Newfoundland	
Light rime accretion areas	2a, 2c, 5, 7a, 7c
Heavy rime accretion areas	2b, 7b
Coastal areas	3, 4, 6, 8a
Highlands/Avalon Peninsula	9, 11

5

- 6
- 7
- 8

Design criteria for ice (glaze and rime), wind, and combined wind/ice loading are:

Loading	lce	Туре	Wind	Combined		
Zone				lce	Туре	Wind
1	50 mm	Glaze	105 km/hr	25 mm	Glaze	60 km/hr
2a	115 mm	Rime	135 km/hr	60 mm	Rime	95 km/hr
2b	135 mm	Rime	135 km/hr	70 mm	Rime	95 km/hr
2c	115 mm	Rime	135 km/hr	60 mm	Rime	95 km/hr
3	50 mm	Glaze	120 km/hr	25 mm	Glaze	60 km/hr
4	50 mm	Glaze	120 km/hr	25 mm	Glaze	60 km/hr
5	115 mm	Rime	150 km/hr	60 mm	Rime	105 km/hr
6	50 mm	Glaze	120 km/hr	25 mm	Glaze	60 km/hr
7a	115 mm	Rime	180 km/hr	60 mm	Rime	125 km/hr
7b	135 mm	Rime	180 km/hr	70 mm	Rime	125 km/hr
7c	115 mm	Rime	180 km/hr	60 mm	Rime	125 km/hr
8a	50 mm	Glaze	120 km/hr	25 mm	Glaze	60 km/hr
8b	50 mm	Glaze	105 km/hr	25 mm	Glaze	60 km/hr
9	75 mm	Glaze	130 km/hr	45 mm	Glaze	60 km/hr
10	50 mm	Glaze	105 km/hr	25 mm	Glaze	60 km/hr
11	75 mm	Glaze	130 km/hr	45 mm	Glaze	60 km/hr

⁵ <u>http://www.pub.nl.ca/applications/MuskratFalls2011/files/exhibits/Exhibit85.pdf</u>, page 35.

Page 13

NP-NLH-004

	Island Interconnected System Supply Issues and Power Outages
_	Page 11 of 57
1	These design criteria, together with topography and routing constraints, including
2	land features, water bodies, structures, and roads, were then used as the basis for
3	structure location selection. The development of each design criterion is discussed
4	in the following sections.
5	
6	It should be understood that the final design for the LITL has greater structural
7	capability than the design criteria alone might indicate. This is the result of the
8	design constraints other than climatic loading ruling the line design by limiting span
9	lengths and subsequently reducing the structure loading. It is therefore important
10	to understand that the 'as-designed' capability of the line must be assessed in order
11	to fully understand its structural capability; these assessments are completed for
12	ice, wind, and combined wind and ice loads in this response.
13	
14	Evaluation of LITL Against CSA Climatic Loadings
15	
16	The following sections of this response compare the design criteria for the LITL as
17	well as its as-designed capability against the applicable meteorological loadings
18	presented in CAN/CSA 22.3 No. 60826.
19	
20	Four criteria are considered:
21	• Glaze ice accretion, applicable in Zones 1, 3, 4, 6, 8, 9, 10, and 11;
22	• Rime ice accretion, applicable in Zones 2, 5, and 7;
23	 Wind loading, applicable to the entire route; and
24	• Combined ice (glaze or rime ice, as applicable) and wind.

Page 14

	NP-NLH-004
-	Island Interconnected System Supply Issues and Power Outages
1	Glaze Ice
2	As will be seen in this section, the as-designed LITL structures will be capable of
3	withstanding 150-year CSA return period glaze ice loadings for the portion of the
4	LITL off the Avalon Peninsula and 500-year CSA return period glaze loadings for the
5	portion on the Avalon Peninsula. ⁶ Manitoba Hydro International's (MHI's)
6	conclusions in their DG3 report are also reviewed in this section.
7	
8	CAN/CSA 22.3 No. 60826 provides a methodology for translating reference glaze ice
9	loadings – predictions made for a 10-metre reference height and for a 50-year
10	return period – to structure loadings for a desired return period. Factors are
11	provided in the standard to adjust for tower/conductor height, ⁷ return period ⁸ , and
12	to adjust for uncertainty from the limited number of observing stations in a given
13	area. ⁹ The steps are followed below to establish CSA structure loadings.
14	
15	It should be noted that when the engineering design criteria for the LITL were
16	developed, the 2006 version of the CSA standard (CAN/CSA No. 22.3 60826:2006)
17	was applicable. This was superseded by the 2010 version during LITL design.
18	
19	As a first step, the reference loadings from the standard were considered. In the
20	following table, reference loadings from the standard are presented for each zone
21	along with the corresponding 50-year structure loads, calculated after the
22	applicable adjustments. ¹⁰

⁶ Considering the 2010 edition of the CSA standard.
⁷ CAN/CSA 22.3 No. 60826:2010, Section 6.3.4.1.
⁸ CAN/CSA 22.3 No. 60826:2010, Table CA.2.
⁹ CAN/CSA 22.3 No. 60826:2010, Section 6.3.4.1.
¹⁰ In the 2006 version of the standard, the structure load is derived by multiplying the reference load by 1.5.

Page 15

CIMFP Exhibit P-03188

			Page 1
Zone	Area	2006 CSA Reference	2006 CSA Structure (50 yr)
1	Inner Labrador	20 mm	30 mm
3	Labrador Coast	25 mm	38 mm
4	Northern Pen.	25 mm	38 mm
	Coast		
6	Northern Peninsula	25 mm	38 mm
8a	Central-West NF	25 mm	38 mm
8b	Central-West NF	25 mm	38 mm
9	Birchy Narrows	25 mm	38 mm
10	Central-East NF	25 mm	38 mm
11	Eastern NF	40 mm	60 mm

			NP-	NLH-004
Island Interconnected S	ystem Supp	oly Issues	and Power	Outages

1

5

Although the standard permits calibration to existing lines with a long history of
 satisfactory performance,¹¹ (which is Hydro's experience for significant portions of
 the LITL route) no reductions to the design loadings were considered in these areas.

Kathleen Jones of the Cold Regions Research and Engineering Laboratory, US Army
Corp of Engineers, was engaged to provide an estimate of extreme loading values
for Newfoundland and Labrador. Her report is available as Exhibit 96 of the
Muskrat Falls review.¹² Her analysis predicted estimated loadings that are lower
than those in the CSA standard, but no reductions to the design loadings were
considered.

12

13Table CA.2 of CAN/CSA No. 22.3 60826-06 provided factors to convert glaze ice loads from14the 50-year reference to other desired return periods. The table is reproduced below and15the factors presented in this table are used to convert 50-year loadings to 150 or 500-year16loadings as applicable:

Return Period (years) ¹³	Factor
150	1.15
500	1.30

¹¹ IEC 60826, Section A.1.2.5.

¹² http://www.pub.nl.ca/applications/MuskratFalls2011/files/exhibits/Exhibit96.pdf.

¹³ 2006 edition factors.

Page 16





Figure 9: Climatic Ice Loads along the HVdc Transmission Line Route compared to the CSA Standard 1:500-year return period limit

9 10 11 12

Figure 4 – Climatic Ice Loads from MHI DG3 Report

- MHI's DG3 analysis indicated that the LCP design glaze ice loadings were equivalent
- 12 to or greater than CSA 1:500-year return period loads as shown in the following
- 13 table:

¹⁴ <u>http://muskratfalls.nalcorenergy.com/wp-content/uploads/2013/03/MHI-Review-October-2012.pdf</u>, Fig. 9. <u>http://www.powerinourhands.ca/pdf/MHI.pdf</u>, page 47.

Page 17

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Section	Area	Glaze Ice	CSA 2006	
		Design	500-yr	
1	Inner Labrador	50 mm	39 mm	
3	Labrador Coast	50 mm	49 mm	
4	Northern Pen. Coast	50 mm	49 mm	
6	Northern Peninsula	50 mm	49 mm	
8a	Central-West NF	50 mm	49 mm	
8b	Central-West NF	50 mm	49 mm	
9	Birchy Narrows	75 mm	49 mm	
10	Central-East NE	50 mm	49 mm	

75 mm

NP-NLH-004 Island Interconnected System Supply Issues and Power Outages

78 mm¹⁵

Page 15 of 57

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T.

The 2006 version of the CSA standard was updated (CAN/CSA No. 22.3 60826:2010), and with the release of the 2010 version of the CSA standard, reference glaze ice loadings for various locations in Newfoundland and Labrador were increased. The 2010 reference glaze ice loads are presented below for Newfoundland and Labrador, as shown in the following extract from Figure CA.10 in the standard:¹⁶



11

Eastern NF

¹⁵ Approximately equivalent.

¹⁶ Contours represent reference ice in mm.

NP-NLH-004

Island Interconnected System Supply Issues and Power Outages

Page 1	6 of	57
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1	The 2006 and 2010 CSA reference glaze ice loadings for each zone are shown below
2	and higher resolution maps are provided in Appendix A of this response. Given the
3	heavier loading on the Avalon Peninsula, Zone 11 (Eastern Newfoundland) was
4	separated into sub-zones with the loading for each segment taken from Figure
5	CA.10: ¹⁷

6

7000	Area	2006 CSA	2010 CSA	
Zone	Alea	Reference	Reference	
1	Inner Labrador	20 mm	25 mm	
3	Labrador Coast	25 mm	30 mm	
4	Northern Pen. Coast	25 mm	30 mm	
6	Northern Peninsula	25 mm	25 mm	
8a	Central-West NF	25 mm	25 mm	
8b	Central-West NF	25 mm	25 mm	
9	Birchy Narrows	25 mm	25 mm	
10-1	Central-East NF	25 mm	25 mm	
10-2	Eastern NF: Gander Lake to Port Blandford	25 mm	25 mm	
11-1	Eastern NF : Port Blandford to Sunnyside	40 mm	32.5 mm	
11-2	Eastern NF : Sunnyside to Whitbourne	40 mm	37.5 mm	
11-3	Eastern NF: Whitbourne to Rod and Gun Club	40 mm	40 mm	
11-4	Eastern NF: Rod and Gun Club to Soldier's Pond	40 mm	40 mm	

7

8

Using the 2010 CSA Reference loads, the 50-year structure loads are:¹⁸

Zone	Area	2010 CSA Reference	2010 CSA Structure (50 yr)
1	Inner Labrador	25 mm	38 mm
3	Labrador Coast	30 mm	45 mm
4	Northern Pen. Coast	30 mm	45 mm
6	Northern Peninsula	25 mm	38 mm
8a	Central-West NF	25 mm	38 mm
8b	Central-West NF	25 mm	38 mm

¹⁷ Subzones 10-2 and 11-4 were created for wind analysis but have the same ice loads as for subzones 10-1 and 11-3 respectively.

¹⁸ Structure loads are obtained by multiplying the reference load by a spatial factor of 1.3 and a height factor of 1.15, as per Clause 6.3.4.1 of the Standard.

Page 19

NP-NLH-00
Island Interconnected System Supply Issues and Power Outage

	Page 17 o			f 57
Zone	Area	2010 CSA Reference	2010 CSA Structure (50 yr)	
9	Birchy Narrows	25 mm	38 mm	
10-1	Central-East NF	25 mm	38 mm	
10-2	Eastern NF: Gander Lake to Port Blandford	25 mm	38 mm	
11-1	Eastern NF : Port Blandford to Sunnyside ¹⁹	32.5 mm	49 mm	
11-2	Eastern NF : Sunnyside to Whitbourne	37.5 mm	56 mm	
11-3	Eastern NF: Whitbourne to Rod and Gun Club	40 mm	60 mm	
11-4	Eastern NF: Rod and Gun Club to Soldier's Pond	40 mm	60 mm	

1

2 The conversion factors for adjusting 50-year reference return periods to longer

3 return periods were also increased over those in the 2006 standard. It is

4 noteworthy that the conversion factors for 150 and 500-year return periods were

5 increased by 4% and 18% respectively in the 2010 standard over the 2006 standard.

6 The factors are shown below:

7

Return Period (years)	2006 Factors	2010 Factors	
150	1.15	1.20	
500	1.30	1.42	

8

9 The glaze ice design criteria, along with 50, 150, and 500-year return period CSA

10 loadings, are compared in the table on the following page.

NP-NLH-004

Island Interconnected System Supply Issues and Power Outages

Page 18 of 57

Zone	Area	LITL Glaze Ice Design Criteria	CSA 50-yr Structure (2010)	CSA 150-yr Structure (2010)	CSA 500-yr Structure (2010)	Notes
1	Inner Labrador	50 mm	38 mm	46 mm		LITL criterion > CSA 150-year
3	Labrador Coast	50 mm	45 mm	54 mm		See discussion below
4	Northern Pen. Coast	50 mm	45 mm	54 mm		See discussion below
6	Northern Peninsula	50 mm	38 mm	46 mm		LITL criterion > CSA 150-year
8a	Central-West NF	50 mm	38 mm	46 mm		LITL criterion > CSA 150-year
8b	Central-West NF	50 mm	38 mm	46 mm		LITL criterion > CSA 150-year
9	Birchy Narrows	75 mm	38 mm	46 mm		LITL criterion > CSA 150-year
10-1	Central-East NF	50 mm	38 mm	46 mm		LITL criterion > CSA 150-year
10-2	Eastern NF: Gander Lake to Port Blandford	50 mm	38 mm	46 mm		LITL criterion > CSA 150-year
11-1	Eastern NF : Port Blandford to Sunnyside	75 mm	49 mm		70 mm	LITL criterion > CSA 500-year
11-2	Eastern NF : Sunnyside to Whitbourne	75 mm	56 mm		80 mm	See discussion below
11-3	Eastern NF: Whitbourne to Rod and Gun Club	75 mm	60 mm		85 mm	See discussion below
11-4	Eastern NF: Rod and Gun Club to Soldier's Pond	75 mm	60 mm		85 mm	See discussion below

NP-NLH-004

Island Interconnected System Supply Issues and Power Outages Page 19 of 57 1 In Zones 3 and 4, the structures would be required to withstand a design loading of 2 54 mm rather than 50 mm to achieve the desired 150-year design return period. The as-designed structures in Zones 3 and 4 were re-analyzed at 54 mm²⁰ radial 3 glaze ice loads, and all structures were confirmed to be loaded to less than their 4 5 design capabilities, thus indicating that they are designed in a manner to withstand 6 glaze ice loads in excess of CSA 150-year loadings. 7 8 Further analysis was also undertaken to verify the as-designed structural capacity of the LITL in Zones 11-2, 11-3, and 11-4. The as-designed structures were re-analyzed 9 10 at the noted CSA 500-year loading, and all structures were confirmed to be loaded 11 to less than their design capabilities, thus indicating this section of the LITL is 12 designed in a manner to exceed 500-year CSA return period loads. 13 14 The utilization of the LITL structures for glaze ice loads is shown in the following two 15 charts. The horizontal axis represents the LITL structure number – 3,233 structures 16 from Muskrat Falls to Soldiers Pond; the vertical axis shows the structural utilization 17 of each structure. Structural utilization greater than 100% indicates the structure is 18 being loaded beyond its design capability, and less than 100% indicates it is capable 19 of withstanding the imposed load (or more). The first chart is for the 150-year glaze 20 zones and the second is for the 500-year Zone 11. 21 22 The as-designed analysis confirms the ability of the LITL to withstand CSA 150-year 23 glaze ice loadings in zones outside the Avalon Peninsula and CSA 500-year glaze ice 24 loadings on the Avalon Peninsula.

²⁰ CSA 150-year loading

Page 20 of 57



NP-NLH-004 Island Interconnected System Supply Issues and Power Outages

Page 21 of 57



NP-NLH-004

Island Interconnected System Supply Issues and Power Outages

Page 22 of 57

Zone	Area	LITL Glaze Ice Design Criteria	CSA 50-yr Structure (2010)	CSA 150-yr Structure (2010)	CSA 500-yr Structure (2010)	Notes
1	Inner Labrador	50 mm	38 mm	46 mm		LITL criterion > CSA 150-year
3	Labrador Coast	50 mm	45 mm	54 mm		LITL as-designed capability confirmed to be in excess of CSA 150-year loading
4	Northern Pen. Coast	50 mm	45 mm	54 mm		LITL as-designed capability confirmed to be in excess of CSA 150-year loading
6	Northern Peninsula	50 mm	38 mm	46 mm		LITL criterion > CSA 150-year
8a	Central-West NF	50 mm	38 mm	46 mm		LITL criterion > CSA 150-year
8b	Central-West NF	50 mm	38 mm	46 mm		LITL criterion > CSA 150-year
9	Birchy Narrows	75 mm	38 mm	46 mm		LITL criterion > CSA 150-year
10-1	Central-East NF	50 mm	38 mm	46 mm		LITL criterion > CSA 150-year
10-2	Eastern NF: Gander Lake to Port Blandford	50 mm	38 mm	46 mm		LITL criterion > CSA 150-year
11-1	Eastern NF: Port Blandford to Sunnyside	75 mm	49 mm		70 mm	LITL criterion > CSA 500-year
11-2	Eastern NF: Sunnyside to Whitbourne	75 mm	56 mm		80 mm	LITL as-designed capability confirmed to be in excess of CSA 500-year loading
11-3	Eastern NF: Whitbourne to Rod and Gun Club	75 mm	60 mm		85 mm	LITL as-designed capability confirmed to be in excess of CSA 500-year loading
11-4	Eastern NF: Rod and Gun Club to Soldier's Pond	75 mm	60 mm		85 mm	LITL as-designed capability confirmed to be in excess of CSA 500-year loading

Page 25

	NP-NLH-004 Island Interconnected System Supply Issues and Power Outages
_	Page 23 of 57
1	Rime Ice
2	The ability of the LITL structures to withstand rime ice accretion in areas subject to
3	this phenomenon is discussed in this section. As will be seen below, the as-
4	designed LITL structures in these areas will be capable of withstanding 500-year
5	loads as predicted by applicable meteorological models.
6	
7	Rime ice accretion has long been recognized as a design consideration for portions
8	of the LITL route, particularly in the Long Range Mountains. Data collection has
9	been undertaken over many years to inform the development of suitable criteria
10	for this area. As CAN/CSA 22.3 No. 60826 does not address rime or in-cloud ice
11	accumulation, Landsvirkjun Power (LVP) was engaged to develop a rime ice
12	accretion model to predict rime ice accumulation in these areas based on historic
13	data recorded from nearby weather stations, and to advise on the selected routing
14	for the transmission line in the Long Range Mountains and also in areas in Labrador
15	where rime icing occurs.
16	
17	Test structures and spans were established in the mid 1970's, and two
18	instrumented test spans measuring meteorological and load data were installed in
19	2009 (2009-1 and 2009-2) and one original test span from the 1970's was
20	instrumented in 2010 (2010-1). The data record collected from physical observation
21	of the test towers from 1975 to 1985, and the electronic data from the
22	instrumented test spans were used in an extensive study by LVP using numerical
23	models, namely Weather Observation Model (WOBs) and Weather Research and
24	Forecasting (WRF).
25	
26	The WOBs model was used to analyze and determine the 50-year icing levels for
27	both the 2009-1 and 2009-2 sites. The WRF model was used to predict rime ice
28	accretion throughout the Long Range Mountains corridor. From the WRF model

Page 26

				NP-NLH-004			
	Island Interconnected System Supply Issues and Power Outages						
				Page 24 of 57			
and the ty	pical result displayed i	n Figure 6 below	, it was obvious tha	at the worst area			
of exposu	re and rime ice load wa	as in the area of	test span 2009-1.				
In the loca	ation of Test Span 2009	9-1, the 50-year r	ime ice load was de	etermined to be			
40 kg/m, d	or 135 mm of radial rin	ne ice. The 50-ye	ar icing levels for 2	009-2 were			
determine	ed to be 9 kg/m, or 52 r	mm of radial rim	e, only 22% of the	weight per			
metre ind	icated for the 2009-1 lo	ocation.					
As indicate as follows	ed in Table 8 of the rep :	oort, ²¹ reference	e rime ice loadings v	were predicted			
Test Span	Elevation	25-year	50-year	150-year			
2009-1	600 m	35 kg/m	40 kg/m	47 kg/m			
2009-2	530 m	8 kg/m	9 kg/m	11 kg/m			

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In radial measurements, the corresponding loadings are:

14

Test Span	Elevation 25-year		50-year	150-year	
2009-1	600 m	124 mm	135 mm	147 mm	
2009-2	530 m	49 mm	52 mm	60 mm	

15

Given that rime ice is most prominent in exposed areas, or elevations that have a higher cloud exposure, the WRF model results were used to aid in line routing by selecting locations that are not exposed to high levels of rime ice, such as areas that are sheltered by hills, or areas of lower elevation. As shown in Figure 6 below, no area of the line route is near the level of icing experienced by 2009-1, and in fact, the predicted 50-year reference rime accumulation for the route through the Long Range Mountains is consistent with test span 2009-2, or 52 mm. The corresponding

²¹ <u>http://www.pub.nl.ca/applications/MuskratFalls2011/files/exhibits/Exhibit95.pdf</u>, page 49.

Page 27

NP-NLH-004

	Island Interconnected System Supply Issues and Power Outages
	Page 25 of 57
1	accumulation at structure and conductor height is 60 mm, with application of the
2	1.15 height adjustment. ²²
3	
4	Given the remote nature of the area and concerns about access to this area during
5	the winter period, the heavier rime ice load was maintained as a design load.
6	Although Zone 7b experiences rime ice near the levels of test span 2009-2 (9 kg/m,
7	or 52 mm radial rime), the rime ice level of 2009-1 were used for this area, given
8	that it is between 550 m and 600 m elevation. The remainder of the Long Range
9	Mountains used a reduced level of 115 mm radial Rime (31 kg/m), but this value is
10	still significantly higher than the model indicated for the selected line route.
11	
12	As further security, given the observation that rime ice will shed off the conductors
13	but not necessarily off the tower, given the lattice tower layout, towers were
14	designed to be completely encased in rime ice, with no voids. Therefore, instead of
15	just using an elevated icing level on the tower, it was assumed that the tower would
16	carry the full icing load over the whole structure.
17	
18	The results from the test spans demonstrate the value of routing the line inland and
19	through valleys at lower elevations. Using Figure 56 from Exhibit 95 as a basis,
20	Figure 6 below shows the test span locations, the transmission line study corridors
21	(bounded by red lines), and the final transmission line route (shown as the yellow
22	line within the eastern corridor).

²² Given that the WRF model is capable of identifying the area of highest accretion, the CSA spatial factor is not applicable.

Page 28



NP-NLH-004 Island Interconnected System Supply Issues and Power Outages

Figure 6 – Rime Ice Accretion in Long Range Mountain Crossing

3	Section A.5.6.1 of IEC 60826 makes a similar observation:
4 5 6 7 8 9 10 11	The regional and local topography (large and medium scale) modifies the vertical motions of the atmosphere and hence the cloud structure and icing. Coastal mountains along the windward side of the continents act to force moist air upwards, leading to a cooling of the air with condensation of water vapour and droplet growth, eventually with precipitation. The most severe in-cloud icing occurs above the condensation level and the freezing level on freely exposed heights, where mountain valleys force moist air through passes and thus both lift the air and strengthen the wind.
12 13 14 15 16 17 18	On the leeward side of the mountains the descent of an air mass results in internal heating of the air and evaporation of droplets, eventually with a total dissolution of clouds. A local shelter of hills not more than 50 m higher on the windward side may give a significant reduction in ice loadings. For this reason, routes in high mountains may very well be suited for overhead lines, provided they are sheltered against icing wind directions.

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Page 29

NP-NLH-004 Island Interconnected System Supply Issues and Power Outages

Page 27 of 57

- 1 The design criteria, along with the 50, 150, and 500-year structure loadings
- 2 predicted by the models are presented in the following table:
- 3

Zone	Area	LITL Rime Design Criteria	WOBS/WRF 50-year Structure	WOBS/WRF 150-year Structure	WOBS/WRF 500-year Structure
7a	Long Range Mountains	115 mm	60 mm	72 mm	85 mm
7b	Long Range Mountains	135 mm	60 mm	72 mm	85 mm
7c	Long Range Mountains	115 mm	60 mm	72 mm	85 mm

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7

To summarize, for the Long Range Mountains, the design loads are between 35% and 58% greater than those predicted by the meteorological models, with an estimated return period in excess of 500 years.

8

9 Two other rime ice zones were identified along the LITL corridor; in coastal 10 Labrador north of the Strait of Belle Isle, and on the Great Northern Peninsula at 11 the Highlands of St. John. Analysis by Landsvirkjun also took these areas into 12 account.

13

14 For southeastern Labrador, WOBs model analysis was used to determine the rime 15 ice and wind speeds, with a central area (2b) most affected by heavy rime ice 16 (design established at 135 mm) and extended areas (2a & 2c) affected by lower 17 rime ice loads (design established at 115 mm). These zones are significantly lower 18 in elevation at less than 450 m than the Long Range Mountains (LRM) crossing. 19 Therefore, a particular icing event was calculated to produce a significantly lower 20 rime ice load; however persistence, or the reduced frequency of rime ice shedding off of the conductor given the colder temperatures, means that an overall design 21 22 rime ice load similar to the LRM crossing was considered prudent. The WRF model 23 was also used in this area as a route selection aid; however, the key was to avoid 24 the areas of higher elevation and significant exposure.

Page 30

NP-	NLH-	004
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Island Interconnected System Supply Issues and Power Outages

	Page 28 of 57
1	The Highlands of St. John were also analyzed in the same way. Areas of
2	environmental sensitivity and local topography meant that only two routes were
3	available for selection, with the first in a valley east of the Highlands of St. John, and
4	the second on a ridge just east of the valley. The valley was quickly selected due to
5	the significant load reduction because of sheltering effects. The Highlands of St.
6	John reach an elevation of 600 m; however, the valley where the line route was
7	selected reaches a maximum elevation of 500 m. This significant sheltering effect
8	and the research in the icing already determined for the LRM led to selection of a
9	rime ice load of 115 mm.
10	
11	In both southeastern Labrador and the Highlands of St. John, rime ice loadings are
12	expected to be consistent with those predicted by the models for Long Range
13	Mountain site 2009-2.
14	
15	In each of the rime ice cases, design criteria are significantly greater than the 500-
16	year loadings predicted by the rime ice accretion models. Given the remoteness of
17	these locations, this was considered to be an appropriate design decision.

NP-NLH-004

Island Interconnected System Supply Issues and Power Outages

Page 29 of 57

		LITL Rime	WOBS/WRF	WOBS/WRF	WOBS/WRF	
Zone	Area	Design	50-year	150-year	500-year	Notes
		Criteria	Structure	Structure	Structure	
2a	Alpine Labrador	115 mm	60 mm	72 mm	85 mm	LITL criterion > 500-year model
2b	Alpine Labrador	135 mm	60 mm	72 mm	85 mm	LITL criterion > 500-year model
2c	Alpine Labrador	115 mm	60 mm	72 mm	85 mm	LITL criterion > 500-year model
5	Highlands of St. John	115 mm	60 mm	72 mm	85 mm	LITL criterion > 500-year model
7a	Long Range Mountains	115 mm	60 mm	72 mm	85 mm	LITL criterion > 500-year model
7b	Long Range Mountains	135 mm	60 mm	72 mm	85 mm	LITL criterion > 500-year model
7c	Long Range Mountains	115 mm	60 mm	72 mm	85 mm	LITL criterion > 500-year model

Page 32

NP-NLH-004
Island Interconnected System Supply Issues and Power Outages
Page 30 of 57
Wind
As will be seen below, the as-designed LITL structures will be capable of
withstanding wind loads on the Avalon Peninsula consistent with 500-year return
period CSA loadings and with at least 150-year return period CSA wind loadings for
the remainder of the route. The structures will be capable of withstanding greater

6 than 500-year return period CSA wind loadings in rime ice areas.

8 The CAN/CSA C22.3 No. 60826 standard was also considered as a basis for wind
9 loading. Figure 7 presents reference wind speeds for Newfoundland, and Figure 8
10 presents reference wind speeds for Labrador.



Figure 7 – Figure CA.1 from CAN/CSA C22.3 No. 60826

Page 33

CIMFP Exhibit P-03188



NP-NLH-004 Island Interconnected System Supply Issues and Power Outages

Figure 8 – Figure CA.3 from CAN/CSA C22.3 No. 60826

Higher resolution versions of these maps are provided in Appendix B of this response.

4 5

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- 6 Reference wind speeds are 10-minute average wind speeds (not gusts), measured
- 7 at 10 m above ground, and are provided for flat, open terrain.

Page 34

NP-NLH-004

Island Interconnected System Supply Issues and Power Outages

Page 32 of 57

1

The 50-year reference loadings²³ for each zone are as follows:

2

Zone	Area	CSA 50-yr Reference
1	Inner Labrador	100 km/hr
2a	Alpine Labrador	100 km/hr
2b	Alpine Labrador	100 km/hr
2c	Alpine Labrador	110 km/hr
3	Labrador Coast	120 km/hr
4	Northern Pen. Coast	120 km/hr
5	Highlands St. John	110 km/hr
6	Northern Peninsula	110 km/hr
7a	Long Range Mountains	110 km/hr
7b	Long Range Mountains	110 km/hr
7c	Long Range Mountains	110 km/hr
8a	Central-West NF	110 km/hr
8b	Central-West NF	100 km/hr
9	Birchy Narrows	100 km/hr
10-1	Central-East NF	100 km/hr
10-2	Eastern NF	110 km/hr
11-1,2,3	Western Avalon	125 km/hr
11-4	Eastern Avalon	130 km/hr

3

Terrain and obstacles at ground level are important factors in determining the
effects of wind. This is discussed in the CSA standard and the underlying IEC
standard – IEC 60826 2003:

8	Wind speed and turbulence depends on the terrain roughness. With
9	increasing terrain roughness, turbulence increases and wind speed decreases
10	near ground level. Four types of terrain categories, with increasing
11	roughness values, are considered in this standard as indicated in Table 4.

²³ The structure wind loadings are the same as the reference loadings with no adjustments required.

Page 35

CIMFP Exhibit P-03188

NP-NLH-004 Island Interconnected System Supply Issues and Power Outages

Page 33 of 57

Table 4 – Classification of terrain categories

Terrain category	Roughness characteristics	κ _R
A	Large stretch of water upwind, flat coastal areas	1,08
В	Open country with very few obstacles, for example airports or cultivated fields with few trees or buildings	1,00
С	Terrain with numerous small obstacles of low height (hedges, trees and buildings)	0,85
D	Suburban areas or terrain with many tall trees	0,67

1

The reference wind speeds in the CSA standard correspond to Terrain Category B
 conditions. The vast majority of the LITL route is through forested or hilly terrain
 rather than through open country as might be expected at an airport. Design
 calculations consider these effects, and therefore Terrain Category C was selected
 for all glaze ice zones. As a conservative assumption, line designers maintained
 Terrain Category B in the rime ice zones, even though topography suggests
 Category C. ²⁴

9

10 Based on Table CA.2²⁵ of CAN/CSA 22.3 No. 60826, the adjustment factors to

11 modify the 50-year reference for wind to 150-year and 500-year return periods are

12 1.10 and 1.20 respectively.²⁶ The LITL wind design criteria, along with the 50, 150,

13 and 500-year wind speeds are presented in the following table.

 $^{^{24}}$ The K_R factor in the IEC table is multiplied by the reference wind speed, so structures in Terrain Category C see lower effective wind speeds than those in Category B.

²⁵ Table CA.2 provides conversion factors for both ice and wind. The ice factors were referenced previously.

NP-NLH-004

Island Interconnected System Supply Issues and Power Outages Page 34 of 57

7	0	LITL Design CSA 50-year CSA		CSA	Netes	
Zone	Area	Criteria	Reference	150-year	500-year	Notes
1	Inner Labrador	105 km/hr	100 km/hr	110 km/hr		See discussion below
2a	Alpine Labrador	135 km/hr	100 km/hr	110 km/hr	120 km/hr	LITL Criterion > CSA 500-year
2b	Alpine Labrador	135 km/hr	100 km/hr	110 km/hr	120 km/hr	LITL Criterion > CSA 500-year
2c	Alpine Labrador	135 km/hr	110 km/hr	121 km/hr	132 km/hr	LITL Criterion > CSA 500-year
3	Labrador Coast	120 km/hr	120 km/hr	132 km/hr		See discussion below
4	Northern Pen. Coast	120 km/hr	120 km/hr	132 km/hr		See discussion below
5	Highlands St. John	150 km/hr	110 km/hr	121 km/hr	132 km/hr	LITL Criterion > CSA 500-year
6	Northern Peninsula	120 km/hr	110 km/hr	121 km/hr		See discussion below
7a	Long Range Mountains	180 km/hr	110 km/hr	121 km/hr	132 km/hr	LITL Criterion > CSA 500-year
7b	Long Range Mountains	180 km/hr	110 km/hr	121 km/hr	132 km/hr	LITL Criterion > CSA 500-year
7c	Long Range Mountains	180 km/hr	110 km/hr	121 km/hr	132 km/hr	LITL Criterion > CSA 500-year
8a	Central-West NF	120 km/hr	110 km/hr	121 km/hr		See discussion below
8b	Central-West NF	105 km/hr	100 km/hr	110 km/hr		See discussion below
9	Birchy Narrows	130 km/hr	100 km/hr	110 km/hr	120 km/hr	LITL Criterion > CSA 500-year
10-1	Central-East NF	105 km/hr	100 km/hr	110 km/hr		See discussion below
10-2	Eastern NF	105 km/hr	110 km/hr	121 km/hr		See discussion below
11-1	Eastern NF	130 km/hr	125 km/hr		150 km/hr	See discussion below
11-2,3	Avalon Peninsula	130 km/hr	125 km/hr		150 km/hr	See discussion below
11-4	Eastern Avalon	130 km/hr	130 km/hr		156 km/hr	See discussion below

Page 37

NP-NLH-004 Island Interconnected System Supply Issues and Power Outages Page 35 of 57 The LITL design criteria exceed the CSA 500-year wind criteria in rime ice zones and Zone 9, and are approximately the same as 150-year criteria in several zones. In order to confirm the LITL's ability to withstand greater wind loading, the asdesigned structures were subjected to CSA 150-year winds in zones 1, 3, 4, 6, 8, and 10. 500-year winds were applied to Zone 11. The results are shown on the charts on the following pages.

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Page 36 of 57



Page 37 of 57



NP-NLH-004

Island Interconnected System Supply Issues and Power Outages Page 38 of 57

7000	Aroa	LITL Design CSA 50-year		CSA	CSA	Notos	
Zone	Area	Criteria	Reference	150-year	500-year	Notes	
1	Inner Labrador	105 km/hr	100 km/hr	110 km/hr		LITL capability confirmed in excess	
						of CSA 150-year loading	
2a	Alpine Labrador	135 km/hr	100 km/hr	110 km/hr	120 km/hr	LITL Criterion > CSA 500-year	
2b	Alpine Labrador	135 km/hr	100 km/hr	110 km/hr	120 km/hr	LITL Criterion > CSA 500-year	
2c	Alpine Labrador	135 km/hr	110 km/hr	121 km/hr	132 km/hr	LITL Criterion > CSA 500-year	
3	Labrador Coast	120 km/hr	120 km/hr	132 km/hr		LITL capability confirmed in excess	
						of CSA 150-year loading	
4	Northern Pen. Coast	120 km/hr	120 km/hr	132 km/hr		LITL capability confirmed in excess	
						of CSA 150-year loading	
5	Highlands St. John	150 km/hr	110 km/hr	121 km/hr	132 km/hr	LITL Criterion > CSA 500-year	
6	Northern Peninsula	120 km/hr	110 km/hr	121 km/hr		LITL capability confirmed in excess	
						of CSA 150-year loading	
7a	Long Range Mountains	180 km/hr	110 km/hr	121 km/hr	132 km/hr	LITL Criterion > CSA 500-year	
7b	Long Range Mountains	180 km/hr	110 km/hr	121 km/hr	132 km/hr	LITL Criterion > CSA 500-year	
7c	Long Range Mountains	180 km/hr	110 km/hr	121 km/hr	132 km/hr	LITL Criterion > CSA 500-year	
8a	Central-West NF	120 km/hr	110 km/hr	121 km/hr		LITL capability confirmed in excess	
						of CSA 150-year loading	
8b	Central-West NF	105 km/hr	100 km/hr	110 km/hr		LITL capability confirmed in excess	
						of CSA 150-year loading	
9	Birchy Narrows	130 km/hr	100 km/hr	110 km/hr	120 km/hr	LITL Criterion > CSA 500-year	
10-1	Central-East NF	105 km/hr	100 km/hr	110 km/hr		LITL capability confirmed in excess	
						of CSA 150-year loading	
10-2	Eastern NF	105 km/hr	110 km/hr	121 km/hr		LITL capability confirmed in excess	
						of CSA 150-year loading	
11-1	Eastern NF	130 km/hr	125 km/hr		150 km/hr	LITL capability confirmed in excess	
						of CSA 500-year loading	
11-2,3	Avalon Peninsula	130 km/hr	125 km/hr		150 km/hr	LITL capability confirmed in excess	
						of CSA 500-year loading	
11-4	Eastern Avalon	130 km/hr	130 km/hr		156 km/hr	LITL capability confirmed in excess	
						of CSA 500-year loading	

Page 41

	NP-NLH-004
-	Island Interconnected System Supply Issues and Power Outages
1	The as-designed analysis confirmed that no structure in zones off the Avalon
2	Peninsula exceeds its design capacity when subjected to CSA 150-year wind loads
3	and that no structure in Zone 11 on the Avalon Peninsula exceeds its design
4	capacity when subjected to CSA 500-year wind loads.
5	
6	As previously indicated, the design criteria in rime ice zones are greater than CSA
7	500-year return period winds.
8	
9	Combined Wind and Ice Loading
10	Combined wind and ice loading is recognized as a design factor, and combined
11	design criteria were established for the LITL as outlined in Table 1. These have been
12	developed from operational experience and also from an assessment of measured
13	wind conditions during icing events, including wind speed measurements taken in
14	the Long Range Mountains.
15	
16	Neither CAN/CSA 22.3 No. 60826 nor the underlying IEC standard have an
17	established methodology for addressing combined wind and ice loading design
18	criteria. A proposed approach is presented in the IEC standard, as quoted below:
19	
20 21 22 23	6.4.1 Combined probabilities – Principle proposed The action of wind on ice-covered conductors involves at least three variables: wind speed that occurs in presence with icing, ice weight and ice shape (effect of drag
24 25	coefficient). This action results in both transversal and vertical loads.
26 27 28 29 30 31	Ideally, statistics of wind speed during ice presence on conductors should be used to generate the combined loadings of ice and wind corresponding to the selected reliability level. Since detailed data and observations on ice weight, ice shape and coincident wind, are not commonly available, it is proposed to combine these variables in such a way that the resulting load combinations will have the same return periods T as those adopted for each reliability level.

Page 42

	NP-NLH-004
-	Island Interconnected System Supply Issues and Power Outages
	Page 40 of 57
1	It is noteworthy that this section of the standard has remained outstanding in both
2	the 2006 and 2010 editions of the standard, and has not been established as a
3	design requirement.
4	
5	Given the importance of the LITL, the as-designed structures were tested against
6	conditions developed using the proposed methodology in the standard in order to
7	confirm the capability of the line to combined wind and ice conditions.
8	
9	The proposed principle considers two combined loading scenarios for the line:
10	a) An extreme wind event combined with average maximum (high probability)
11	icing conditions, and
12	b) A maximum ice event (at maximum design return period) combined with
13	average maximum (high probability) wind conditions.
14	Reference wind speeds are reduced to represent typical maximum wind speeds
15	during icing conditions and also the relative rarity of maximum wind speeds during
16	icing events. The meteorological conditions used ²⁷ are:
17	
18	Case a) – Extreme Wind + Ice
19	150-year zones, 0.72 x reference wind speed
20	500-year zones, 0.85 x reference wind speed
21	Icing 0.40 x reference ice loading
22	
23	Case b) – Maximum Ice + Wind
24	Icing 150 or 500-year ice loading, as applicable
25	Wind 0.40 x reference wind speed

²⁷ From IEC 60826, Clause 6.4.2 and 6.4.4.1.

Page 43

NP-NLH-004 Island Interconnected System Supply Issues and Power Outages Page 41 of 57

- 1 Applying these factors results in IEC Extreme Wind + Ice and IEC Maximum Ice +
- 2 Wind combined wind-ice conditions for each zone. These combined loadings are
- 3 presented in the following table:

Page 44

NP-NLH-004

Island Interconnected System Supply Issues and Power Outages Page 42 of 57

		LCP Combi	ned Wind/Ice	IEC Extrer	ne Wind + Ice	IEC Maximum Ice + Wind	
Zone	туре	lce	Wind	lce	Wind	lce	Wind
1	Glaze	25 mm	60 km/hr	15 mm	72 km/hr	46 mm	45 km/hr
2a	Rime	60 mm	95 km/hr	24 mm	72 km/hr	72 mm	45 km/hr
2b	Rime	70 mm	95 km/hr	24 mm	72 km/hr	72 mm	45 km/hr
2c	Rime	60 mm	95 km/hr	24 mm	79 km/hr	72 mm	50 km/hr
3	Glaze	25 mm	60 km/hr	18 mm	86 km/hr	54 mm	48 km/hr
4	Glaze	25 mm	60 km/hr	18 mm	86 km/hr	54 mm	54 km/hr
5	Rime	60 mm	105 km/hr	24 mm	86 km/hr	72 mm	54 km/hr
6	Glaze	25 mm	60 km/hr	15 mm	86 km/hr	46 mm	54 km/hr
7a	Rime	60 mm	125 km/hr	24 mm	79 km/hr	72 mm	50 km/hr
7b	Rime	70 mm	125 km/hr	24 mm	79 km/hr	72 mm	50 km/hr
7c	Rime	60 mm	125 km/hr	24 mm	79 km/hr	72 mm	50 km/hr
8a	Glaze	25 mm	60 km/hr	15 mm	76 km/hr	46 mm	47 km/hr
8b	Glaze	25 mm	60 km/hr	15 mm	72 km/hr	46 mm	45 km/hr
9	Glaze	45 mm	60 km/hr	15 mm	72 km/hr	46 mm	45 km/hr
10-1	Glaze	25 mm	60 km/hr	15 mm	72 km/hr	46 mm	45 km/hr
10-2	Glaze	25 mm	60 km/hr	15 mm	79 km/hr	46 mm	50 km/hr
11-1	Glaze	45 mm	60 km/hr	20 mm	86 km/hr	70 mm	48 km/hr
11-2	Glaze	45 mm	60 km/hr	22 mm	86 km/hr	80 mm	48 km/hr
11-3	Glaze	45 mm	60 km/hr	24 mm	90 km/hr	85 mm	50 km/hr
11-4	Glaze	45 mm	60 km/hr	24 mm	94 km/hr	85 mm	52 km/hr

Page 45

NP-NLH-004 Island Interconnected System Supply Issues and Power Outages
Page 43 of 57
The as-designed structures were then subjected to the combined wind/ice
conditions as presented above. In all three cases, no exceedances of the structural
capacity of any structure on the line occurred.
Structural utilization for the IEC combined ice/wind cases are shown on the
following pages.
No structure was loaded beyond 100% of its structural capacity in any of these
scenarios, thus confirming the capacity of all structures to withstand the proposed
IEC 60826 combined wind and ice standards for both extreme wind and ice
scenarios.

Page 44 of 57



NP-NLH-004 Island Interconnected System Supply Issues and Power Outages

Page 45 of 57



Page 46 of 57



Page 47 of 57



Page 48 of 57



Page 49 of 57



Page 52

	NP-NLH-004
_	Island Interconnected System Supply Issues and Power Outages
1	Summary and Conclusions:
-	The design criteria for the LITL were developed following the principles outlined in
2	CAN/CSA 22.3 No. 60826 They were developed following the principle's outlined in
2	data from the standard, operational experience and identified operational risks, as
4	uata nom the standard, operational experience and identified operational risks, as
5	well as model data for unique conditions (rime ice) not incorporated in the
6	standard.
7	
8	In addition to the design criteria established for the LITL, the as-designed structures
9	were evaluated against the following loadings:
10	a) CSA 150-year ice loadings for the line section off the Avalon Peninsula.
11	b) CSA 500-year ice loadings for the route on the Avalon Peninsula.
12	c) CSA 150-year wind loadings for the line section off the Avalon Peninsula.
13	d) CSA 500-year wind loadings for the line section on the Avalon Peninsula.
14	No structure was loaded beyond 100% of its design structural capacity in any of
15	these scenarios.
16	
17	The design criteria for rime ice accretion are beyond 500-year return period loads
18	predicted by rime ice accretion models developed by LVP, and the design criteria
19	for wind are beyond 500-year CSA return period winds in rime zones.
20	
21	The as-designed structures were evaluated against proposed combined wind and
22	ice criteria in IEC Standard 60826 in the following conditions:
23	a) Extreme wind (72% or 85% of reference wind as applicable for 150-year
24	and 500-year zones respectively) combined with 40% of reference ice
25	load; and
26	b) Maximum ice (150 or 500-year loads as applicable for 150-year and 500-
27	vear zones respectively) combined with 40% of reference wind speed
	, e

Page 53

	NP-NLH-004
	Island Interconnected System Supply Issues and Power Outages
	Page 51 of 57
1	No structure was loaded beyond 100% of its as-designed structural capability in any
2	of these scenarios.
3	
4	In conclusion, the LITL has been designed in accordance with applicable standards
5	and it meets climatic loading conditions commensurate with its important role in
6	the Island Interconnected System.

Page 54



Page 52 of 57



Page 55



NP-NLH-004 Island Interconnected System Supply Issues and Power Outages

Page 56



NP-NLH-004 Island Interconnected System Supply Issues and Power Outages Page 54 of 57

Page 57





Page 58



Page 59



APPENDIX A – Ice Loading Region Maps

Appendix A updated with current meteorological loadings. Loadings are highlighted in each zone

Muskrat Falls Project - Exhibit 97, Appendix A Revision 1 Page 61 Page 16 of 23





Muskrat Falls Project - Exhibit 97, Appendix A Revision 1 Page 17 of 23 Page 62





Muskrat Falls Project - Exhibit 97, Appendix A Revision 1 Page 18 of 23 Page 63



Zone 3 – Labrador Coast

Average Meteorological Loading Zone

Maximum Ice: 50 mm (Glaze), Maximum Wind: 120 km/h, Combined Ice and Wind: 25 mm (Glaze) and 60 km/h

6

LOWER CHURCHILL PROJECT



Zone 4 – Northern Peninsula Coast

Average Meteorological Loading Zone

Maximum Ice: 50 mm (Glaze), Maximum Wind: 120 km/h, Combined Ice and Wind: 25 mm (Glaze) and 60 km/h

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Muskrat Falls Project - Exhibit 97, Appendix A Revision 1 Page 19 of 23 Page 64



Maximum Ice: 50 mm (Glaze), Maximum Wind: 120 km/h, Combined Ice and Wind: 25 mm (Glaze) and 60 km/h





Muskrat Falls Project - Exhibit 97, Appendix A Revision 1 Page 65 Page 20 of 23



Zone 7b – Long Range Mountains Crossing Extreme Alpine Meteorological Loading Zone (Eastern Corridor Alternative Only) Maximum Ice: 135 mm (Rime), Maximum Wind: 180 km/h, Combined Ice and Wind: 70 mm (Rime) and 125 km/h





Muskrat Falls Project - Exhibit 97, Appendix A Revision 1 Page 21 of 23 Page 66





Muskrat Falls Project - Exhibit 97, Appendix A Revision 1 Page 22 of 23 Page 67



Maximum Ice: 50 mm (Glaze), Maximum Wind: 105 km/h, Combined Ice and Wind: 25 mm (Glaze) and 60 km/h

14

LOWER CHURCHILL PROJECT



Zone 9 – The Birchy Narrows

Alpine Meteorological Loading Zone (Northern Corridor Alternative Only) Maximum Ice: 75 mm (Glaze), Maximum Wind: 130 km/h, Combined Ice and Wind: 45 mm (Glaze) and 60 km/h



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Eastern Meteorological Loading Zone

Maximum Ice: 75 mm (Glaze), Maximum Wind: 130 km/h, Combined Ice and Wind: 45 mm (Glaze) and 60 km/h

